

Supplementary Geotechnical Investigation Report

New High School for Medowie

6 Abundance Road, Medowie NSW 2318

Prepared for: Department of Education

A201024.0124.00_B_v2f | Date: 30 January 2025

T. 1300 796 922 | E. info@ade.group







Document Information

Prepared for: Department of Education Report Name: Supplementary Geotechnical Investigation Report Site Address: 6 Abundance Road, Medowie NSW 2318 Report Reference: A201024.0.124.00_B_v2f Date: 30 January 2025

Document Control

Version	Date Author		Date Author Revision description		Reviewer
v1d	20/12/2024	Gulshan Lakshman	Draft for client review	Antony Tam	
v2d	10/01/2025	Jouad Kanaan	Draft for client review	Antony Tam	
v2f	30/01/2025	Jouad Kanaan	Final	Antony Tam	

For and on behalf of **ADE Consulting Group Pty Ltd**

Prepared by:

Reviewed by:

Jouad Kanaan Senior Associate Geotechnical Engineer, CPEng Antony Tam State Manager - Geotechnical Discipline



Table of Contents

1	Intro	duction5
	1.1	General Information5
	1.2	REF Checklist
	1.3	Scope of Work
2	Back	ground
	2.1	Site Description
	2.2	Project description
	2.3	Regional Geology 11
3	Field	work Results
	3.1	Subsurface Conditions
	3.2	Groundwater
4	Labo	ratory Testing
5	Com	nents and Recommendations14
	5.1	Site Preparation and Excavation14
	5.2	Groundwater Management
	5.3	Foundations
	5.3.1	Pile foundation15
	5.4	Cumulative Impact Assessment 16
	5.5	Mitigation Measures
Со	nclusio	n
6	Refer	ences
7	Limit	ations



List of Tables

Table 1: REF Review Checklist – General Requirements	. 6
Table 2: REF Review Checklist – Soil and Water	. 7
Table 3: Summary of Subsurface Profile	12
Table 4: Groundwater Depths encountered during drilling	12
Table 5: Point Load Strength Index Test for Borehole 205	13
Table 6: Point Load Strength Index Test for Borehole 208	13
Table 7: Summary of Geotechnical Foundation Design Parameters recommended	15
Table 8: Summary of Geotechnical design of bored pile Foundation Design Parameters	15
Table 9: Summary of Mitigation Measures	16
Table 10: Summary of Mitigation Measures (Continued)	17

List of Figures

Figure 1: Survey Plan with Boreholes Indicated (Base Map Source: Nearmap)10	
Figure 2: Extract of eSPADE Geological map by NSW Department of Planning and Environment	

Appendices

Appendix I – Borehole Location Plan	21
Appendix II – Borehole Logs and Explanatory Notes	22
Appendix III – Laboratory Test Results	23



1 Introduction

1.1 General Information

This document has been prepared in accordance with the Guidelines for Division 5.1 assessments by the Department of Planning, Housing and Infrastructure (formerly the Department of Planning and Environment), June 2022). This can be accessed here: Development without consent | Planning (<u>nsw.gov.au</u>).

The Department of Education (DoE) commissioned ADE Consulting Group (ADE) to provide an supplementary Intrusive Geotechnical Investigation (IGI) report in relation to the proposed New High School for Medowie located at 6 Abundance Road, Medowie. The proposed school is located at 6 Abundance Road, Medowie NSW 2318. A Site Plan showing the proposed building footprint and borehole locations is presented in **Appendix I.**

This Supplementary Geotechnical Investigation Report has been prepared to support a Review of Environmental Factors (REF) for the proposed New High School for Medowie (the activity). The purpose of the REF is to assess the potential environmental impacts of the activity prescribed by State Environmental Planning Policy (Transport and Infrastructure) 2021 (T&I SEPP) as "development permitted without consent" on land carried out by or on behalf of a public authority under Part 5 of the Environmental Planning and Assessment Act 1979 (EP&A Act).

The activity is to be undertaken pursuant to Chapter 3, Part 3.4, Section 3.37 of the T&I SEPP. The activity will be carried out at 6 Abundance Street, Medowie (the site).

ADE previously prepared a Preliminary Geotechnical Desktop Study (PGDS) report (Reference no. A201024.0124.00_A_v1f), dated 8 May 2024, and a IGI report (Reference no. A201024.0124.00_A_v1d), dated 7th June 2024 for the proposed school infrastructure activity.

This second round of IGI was carried out on 19th to 22nd and 25th to 27th of November 2024 and consisted of drilling five (5) deep boreholes. The purpose of this report is to assess geotechnical conditions, particularly the existing bedrock levels, within the proposed activity area and provide comments and recommendations on the geotechnical parameters to assist in the slab, footing and pile design of the proposed school building foundation's purpose only.

The report includes the inferred excavation conditions and methodology, groundwater, geotechnical design parameters for the potential foundation system, retaining walls design parameters, on-grade floor slabs, and other geotechnical issues associated with the proposed activity.



1.2 REF Checklist

Table 1 and Table 2 below summarise all relevant REF checklist items addressed in this supplementary IGI Report and provide section references for review.

Table 1: REF Review Checklist – General Requirements

Requir	ement	Y	Ν	N/ A	Comments & Report Reference
Genera	I requirements				
Regula • o	tory requirements Does the IGI include details of: the proposed activity?	\boxtimes			
0	need for the activity?	\boxtimes			
0	relevant planning policies, including relevant indicative layout plans, masterplans, strategic plans or Voluntary Planning Agreements apply to the site?	X			Refer Section 1 'Introduction' of this IGI Report
0	how proposal relates to relevant legislation and policies?	\boxtimes			
0	related approvals required?	\boxtimes			
0	relevant determining authority (i.e. the NSW Department of Education)	\boxtimes			
•	a description of the site (including address and lot/DP) and surrounding environment using text and plans/photos including details the environmental features and planning constraints?				Refer Section 2.1 'Site Description' of this IGI Report
•	an assessment of potential impacts of the proposal?	X			Refer Section 5.4 'Cumulative Impact Assessment' in this IGI Report
•	a statement certifying that the contents are true and correct?				Refer Section 7 'Limitations' of this IGI Report 'This report does not provide a complete assessment of the geotechnical status of the site, and it is limited to the scope defined herein"
•	a statement that the proposed activity qualifies as development without consent?	\boxtimes			Refer Section 1 'Introduction' of this IGI Report
•	a schedule of mitigation measures that are specific and deliverable?	\boxtimes			Refer Section 5.5 'Mitigation Measures' of this IGI Report



Table 2: REF Review Checklist – Soil and Water

Requirement	Y	Ν	N/ A	Comments & Report Reference
Soil and Water				
If the site is mapped as, or has otherwise been identified, as having salinity potential, does the geotechnical report consider impacts from salinity and set out measures to mitigate impacts (i.e. Salinity Management Plan) so that they would not be significant?				Refer ADE's previous Detailed Site Investigation Report 'A101024.0124 Medowie DSI v3' dated 22 January 2025. "The site is mapped as being in a non-saline area according to the NSW Office
If the site is mapped as, or has otherwise been identified as having acid sulfate soils (ASS) potential, does the geotechnical report consider impacts from ASS and set out measures to mitigate impacts (i.e. ASS Management Plan) so that they would not be significant?				of Water." Refer ADE's previous Detailed Site Investigation Report 'A101024.0124 Medowie DSI v3' dated 22 January 2025. The report concluded that based on observations of soil texture and type, site conditions and results from field screening tests, that Actual Acid Sulfate Soils (AASS) and Potential Acid Sulfate Soils (PASS) are not considered likely at the site.



1.3 Scope of Work

In summary, this IGI work generally comprised the following:

- Preparation and approval of a Safety, Health, Environment, and Safe Work Method Statement (SHEWMS) prior to undertaking works
- Prior to commencing the fieldwork, underground services were electromagnetically scanned and identified by an accredited service locator.
- In total, five (5) boreholes identified as BH202, BH203, BH205, BH206 and BH208 were drilled within the proposed activity area. Boreholes were drilled using a solid flight auger to about 10m to 15m before wash boring was used. Locations of the boreholes are presented in **Appendix I** of this report.
- Coring was undertaken from depths of approximately 45m using a HQ size core barrel.
- Standard Penetration Test (SPT) were carried out within the boreholes with the first SPT test completed at 1m with additional SPTs completed at 1.5-meter intervals.
- ADE's geotechnical engineer was present full-time onsite during the fieldwork to set out the borehole locations, direct subcontractors, log the subsurface profile in accordance with Australian Standard AS1726-2017, and collect earth samples for subsequent laboratory testing.
- Upon drilling completion, the boreholes were reinstated by using drilling cuttings obtained from the borehole's excavation.
- Preparation of the borehole logs, attached in **Appendix II** along with Explanatory Notes, which describe the investigation techniques adopted and define the logging terms and symbols used.
- Point Load laboratory testing was conducted on the selected rock samples, attached in Appendix III.
- Preparation of a IGI report for the proposed activity, which includes the findings of fieldwork, interpretation of the subsurface conditions, assessment, and recommendations for the proposed activity.



2 Background

2.1 Site Description

The site has a street address of 6 Abundance Road, Medowie. It is 6.51ha in area, and comprises 1 allotment, legally described as Lot 3 in DP788451.

A large proportion of the site is currently unused and vacant. A small shed structure and caravan are located adjacent to the northern boundary. A cluster of buildings including a single storey dwelling, an outhouse/shed structure and temporary greenhouse are located within the southeastern corner.

The site contains a largely vegetated area to the southwest corner. The site is relatively flat with a gradual fall from west to east toward Abundance Road.

The site has a primary frontage to Abundance Road to the east and Ferodale Road to the north. Abundance Road and Ferodale Road are both classified Local Roads. Medowie Road, approximately 1km east of the site, is a classified Regional Road.

The area surrounding the site mostly consists of industrial, rural residential, educational, and agricultural lands. Adjacent to the northwestern boundary is a Shell petrol station and mechanic garage. Adjacent to the northeastern boundary is a medical health clinic. Across Abundance Road along the eastern boundary are a number of warehouse and light industrial developments. Directly north of the site across Ferodale Road are large lots used for agricultural purposes. Medowie Public School is located on Ferodale Road, to the north west of the site, opposite the Shell petrol station. **Figure 1** presents the current state of the investigation area.





Figure 1: Survey Plan with Boreholes Indicated (Base Map Source: Nearmap)

2.2 Project description

The proposed activity involves the construction of school facilities on the site for the purpose of the New High School for Medowie. The site contains a densely vegetated area to the southwest corner which is identified as land with high biodiversity values corresponding to the areas of remnant native vegetation (PCT 3995 – Hunter Coast Paperbark-Swamp Mahogany Forest). The existing dwelling house and other structures on the site will be demolished as part of the works. No other works are proposed within this area.

The proposed new school will accommodate 640 students in 29 permanent teaching spaces including 3 support teaching spaces across 3-storeys of buildings on the site. The proposed activity be delivered across 1 stage, and will consist of the following:

- 29 permanent teaching spaces including 3 support teaching spaces, to accommodate 640 students, and school hall to accommodate 1,000 students. Approximately 10,500 sqm of GFA is proposed.
- Main vehicular ingress and egress to Ferodale Road to the north, with a new pedestrian and vehicle crossing proposed.
- Main pedestrian access to Abundance Road.
- Kiss and ride, and bus drop and pick up areas to Abundance Road (6 x parallel spaces).
- New pedestrian wombat crossing to Abundance Road
- Approximately 55 x car parking spaces and 3 x accessible car parking spaces.
- Approximately 70 x bicycle parking spaces.



- Block A (Admin) consisting of administration and learning spaces.
- Block B (Foodtech/Workshop) consisting of food technology rooms and workshops.
- Block C (Hall) consisting of school hall to accommodate 1,000 students.
- Central quad, 1 playing field, and 1 sports courtyard.

The proposed school development will include the following spaces: general learning spaces, General support learning spaces, administrative services, staff areas, gym and canteen, library areas for science, wood and metal, food and textiles, health PE, performing arts, additional learning spaces, student amenities, storage, movement (stairs and covered walkways).

2.3 Regional Geology

Reference to the 1:100 000 Newcastle Geological Sheet 9232, first edition from Geological Survey of NSW (1975) indicates that the site is underlain by **Pt (Post)**, part of Tomago Coal measures Group and Permian aged. The 'PT' is referred to as shale, mudstone, sandstone, tuff, and coal. **Figure 2** shows the excerpt of the local geology from the Sydney Geological Map below.



Figure 2: Extract of eSPADE Geological map by NSW Department of Planning and Environment.



3 Fieldwork Results

3.1 Subsurface Conditions

A summary of subsurface materials encountered during the investigation is presented in **Table 3** below. Reference should be made to the attached borehole logs in Appendix II of this report. Mudstone bedrock was encountered at a depth approximately 45m below existing ground level.

Unit	Details	Depth					
		BH202	BH203	BH205	BH206	BH208	
Unit 1	Topsoil: Silty CLAY, brown, black, with rootlets	0.00 - 0.20	0.00 - 0.20	0.00 - 0.20	0.00 - 0.20	0.00 - 0.20	
Unit 2	Residual: Silty CLAY, Stiff to Very stiff, with sand and trace ironstone gravels	0.20 – 24.50	0.20 – 14.95	0.20 – 35.00	0.20 – 14.95	0.20 – 25.00	
Unit 3	Silty / Sandy CLAY: Hard, with extremely weathered siltstone vertical bands	24.50 – 27.20	-	35.00 – 46.73	-	25.0 – 44.60	
Unit 4	MUDSTONE	_	_	46.73- 49.74	_	44.6-48.4	

Table 3: Summary of Subsurface Profile

3.2 Groundwater

Groundwater was observed during solid flight auger drilling in boreholes BH202, BH203, BH205 and BH206 at depths between 5m to 9m (Refer **Table 4**). In BH208, groundwater was not observed during auger drilling, and wash boring commenced at a depth of approximately 11.0m below ground level. Wash boring precludes observation of standing groundwater level due to introduction of water.

Table 4: Groundwater Depths encountered during drilling

Borehole Numbers	BH202	BH203	BH205	BH206
Groundwater Depth (m)	5.0	8.5	9.0	7.5



4 Laboratory Testing

Given this second round of IGI particularly aimed to assess the existing bedrock level and rock strength characteristics, Point Load tests were conducted on selected rock core samples retrieved from the geotechnical boreholes by ADE's NATA accredited laboratory.

The Point Load test results indicated the rock strength as very low to low. The results are summarized in **Table 5** and **Table 6**. The laboratory test reports are enclosed in **Appendix III**. The soil profile materials characteristics should refer to the previous IGI prepared by ADE, reference no. A201024.0124.00_A_v1d), dated 7th June 2024.

Table 5: Point Load Strength Index Test for Borehole 205

Borehole No. 205								
Sample No.	Depth (m)	Description	Test type	Point Load Index Is ₍₅₀₎ (MPa)	Failure Modes			
1	46.84	MUDSTONE	Diametrical	0.20	1			
T	40.84	MODSTONE	Axial	0.29	5			
2	47.84		Diametrical	0.24	1			
Z	47.84	MUDSTONE	Axial	0.28	5			
2	40.07	MUDGTONE	Diametrical	0.16	1			
3	48.67	MUDSTONE	Axial	0.21	5			
	40.26	MUDGTONE	Diametrical	0.49	1			
4	49.26	MUDSTONE	Axial	0.42	5			

Table 6: Point Load Strength Index Test for Borehole 208

	Borehole No. 208								
Sample No.	Depth (m)	Description	Test type	Point Load Index Is ₍₅₀₎ (MPa)	Failure Modes				
1	46.26		Diametrical	0.04	1				
1	46.36	MUDSTONE	Axial	0.04	6				
2	46.69	MUDSTONE	Diametrical	0.05	1				
Z	40.09	WIDDSTONE	Axial	0.04	6				
2	47.20	MUDGTONE	Diametrical	0.06	1				
3	47.39	MUDSTONE	Axial	0.16	6				
4	47.70	MUDGTONE	Diametrical	0.33	1				
4	47.79	MUDSTONE	Axial	0.23	5				
_	40.05	MUDGTONE	Diametrical	0.33	1				
5	48.05	MUDSTONE	Axial	0.28	5				



5 Comments and Recommendations

5.1 Site Preparation and Excavation

The site preparation will involve topsoil stripping and ground levelling for the proposed structures. Based on the available geotechnical investigation findings, excavation during site preparation is expected to encounter a 200mm layer of topsoil (Unit 1), followed by Silty CLAY (Unit 2 and Unit 3) residual soil with sand and gravels up to approximately 45m below existing ground level. Based on the borehole information, MUDSTONE bedrock (Unit 4) was encountered at a depth of approximately 46.73m at borehole BH205 and 44.60m at borehole location BH208 respectively.

According to Site Sections 01-03 on the architectural site section drawing ('Site Sections – Sheet 1', Drawing MHS-NBRS-ZZ-ZZ-DR-A-004001, Rev 2, by NBRS, dated 15 January 2025) the estimated general ground fill to be placed over the site varies between approximately 0 - 1m

It is recommended within the footprint of the proposed structures that Unit 1 material should be removed if encountered. This includes the grubbing of tree roots, if present. If required, the excavated areas can be backfilled with suitably engineered fill layers to the designed subgrade level. The topsoil can be stockpiled on site for future use.

As a mitigation measure (Refer Section 5.5 for further detail), any fill, deleterious/surplus material (if present) such as timber, concrete, rubble, and any other unsuitable materials should be identified and disposed offsite. Re-use of site-won materials must comply with Australian Standard AS3798-2007 "Guidelines on earthworks for commercial and residential developments" and the material should be contamination free. Placement and compaction of engineered fill layers must also be in accordance with Australian Standard AS3798-2007. The standard provides guidance on the specification, execution, and control testing of earthworks and associated site preparation works; and on the interpretation and application of the relevant test methods specified in the AS1289 series of standards.

5.2 Groundwater Management

The result of the intrusive investigation finds that groundwater was encountered at depths ranging from 5m to 9m. Encountering groundwater will depend on the completion of the detailed design and foundation type proposed for the building structures. If shallow foundations are used, groundwater will unlikely be encountered. If deep foundations such as piles are used, groundwater will likely be encountered during piling excavation. A Groundwater Management Plan (GMP) is recommended to be prepared and implemented during the construction phase if deep foundations are to be used.



5.3 Foundations

Based on the geotechnical investigation and lab results the following geotechnical design parameters are provided in **Table 7** can be used for the design of footing foundations.

Subsurface Materials	Unit Weight y (kN/m³)	Undrained Cohesion Su (kPa)	Drained Cohesion C' (kPa)	Friction Angle ダ (°)	Elasticity Modulus E (MPa)	Poisson Ratio v'
Unit 2	20	150	5	27	30	0.3
Unit 3	20	200	10	28	60	0.25
Unit 4	24	200-500	-	30	400	0.20

Table 7: Summary of Geotechnical Foundation Design Parameters recommended

Considering the geotechnical properties of the subsoil layer, a deep foundation is recommended to support the development structures. For deep foundation design, the parameter values presented in **Table 8** are recommended.

5.3.1 Pile foundation

Due to deep seated clay layers, subject to applied loading and settlement consideration, we anticipate that the pile foundation would likely be a suitable foundation option for the site. As the site is near existing residential/commercial developments, the bored piles piling method should be considered compared to driven piles to reduce both noise and vibration impact on the surrounding areas. Driven piles may refuse on gravel layers present within the subgrade (approximately 10-12m bgl). **Table 8** provides the recommended geotechnical design values for bored pile foundation design.

Table 8: Summary of Geotechnical design of bored pile Foundation Design Parameters

Subsurface Materials	Pile Ultimate End Bearing Capacity (kPa)	Pile Shaft Ultimate Bearing Capacity (kPa)			
Unit 2*	1350	50			
Unit 3	1800	70			
Unit 4	2000	125			

Notes:

*A minimum pile embedment depth of 10m has been assumed for the ultimate end bearing capacity of the soil.

A Geotechnical Strength Reduction Factor (φ_{_g}) of 0.45 has been considered suitable for this site in accordance with the guidelines provided in AS2159-2009, which considers several factors, including the extent and quality of geotechnical data available. A higher geotechnical strength reduction factor of up to 0.65 can be adopted if pile testing is carried out.

• The Design Geotechnical strength ($R_{d,g}$) shall be calculated as the design ultimate geotechnical strength ($R_{d,ug}$) multiplied by a geotechnical strength reduction factor($\phi_{_g}$) according to AS2159-2009 equation 4.3.1(2) $R_{d,g} = \phi_{_g} R_{d,ug}$



5.4 Cumulative Impact Assessment

We refer to the results of the limited scope of geotechnical and detailed site investigations undertaken at this school site by ADE (Refer Detailed Site Investigation Report 'A101024.0124 Medowie DSI v3' dated 22 January 2025). The report concluded that based on observations of soil texture and type, site conditions and results from field screening tests, that Actual Acid Sulfate Soils (AASS) and Potential Acid Sulfate Soils (PASS) are not considered likely at the site. Further, the site is mapped as being in a non-saline area according to the NSW Office of Water.

The proposed school infrastructures will likely be supported by typical shallow foundations, bored pile foundations or a combination of both. Construction of these foundation types is unlikely to generate significant noise and vibration, subject to the construction methodology and machinery to be nominated by future construction contractors.

The proposed work area is within the well-developed school property. From a geotechnical perspective, we do not anticipate significant social or visual impact or adverse effects on the existing biodiversity.

5.5 Mitigation Measures

The potential project environmental risks and recommended mitigation measures are summarised in **Table 9** below:

Mitigation Name	Aspect/Section	Mitigation Measure	Reason for Mitigation Measure
Noise and Vibration Monitoring	Noise Monitoring, Vibration, Operational Noise	Shallow foundations and bored pile foundations are unlikely to generate significant noise and vibration	Noise and vibration assessment is outside the scope of the IGI, however use of these techniques can reduce noise and vibration impact on surrounding areas
Groundwater Management	Groundwater	Encountering groundwater will depend on the completion of the detailed design and foundation type proposed for the building structures. If shallow foundations are used, groundwater will unlikely be encountered. If deep foundations such as piles are used, groundwater will likely be encountered during piling excavation. Foundations and piles must be sufficiently dewatered to prevent groundwater infiltration and reduce risk of slope instability. A Groundwater Management Plan (GMP) is recommended to be prepared and implemented during the construction	Reduce risk of slope instability, prevent groundwater infiltration to excavation or piles. Identify procedure for disposal of water

Table 9: Summary of Mitigation Measures



Table 10: Summary of Mitigation Measures (Continued)

Mitigation Name	Aspect/Section	Mitigation Measure	Reason for Mitigation Measure
Settlement analysis	Structural Analysis	After selection of the foundation system, it is recommended to carry out a settlement analysis to confirm the total and differential settlements are within the tolerance.	Analysis to be carried out to determine if total and differential settlements are within the design tolerance
Foundation type	Foundation Design	Due to deep seated clay layers observed on site, subject to applied loading and settlement consideration, we anticipate that the pile foundation would likely be suitable foundation option for the site.	Design must consider applied loading and settlement
Removal of soft and unsuitable soils	Earthworks	All loose/soft soil within the footprint of proposed structures to be removed, including grubbing out of tree roots, if present. These layers may be backfilled with suitably engineered fill layers to the designed subgrade level. Any fill unsuitable for re-use, deleterious/surplus material (if present) such as timber, concrete, rubble, should be identified and disposed off-site.	Must be carried out in accordance with AS3798-2007 "Guidelines on Earthworks for Commercial and Residential Developments"
Foundation validation	Earthworks	Validation of the foundation should be completed by an experienced geotechnical engineer	Identify locations of soft or unsuitable material and remediate prior to backfilling and construction of foundations



Conclusion

No significant adverse ground conditions were observed based on the limited scope of geotechnical investigations undertaken at the proposed work area. The proposed new school should consider the geotechnical recommendations provided in this report during the design phase. Key recommendations are listed below, but not limited for general reference:

- All loose/soft soil within the footprint of the proposed structures to be removed, including grubbing out of tree roots, if present.
- Earthwork should be carried out in accordance with AS3798-2007 "Guidelines on Earthworks for Commercial and Residential Developments".
- Deep bedrock was present at the site and found to be at roughly 45m below ground level and consisted of low strength MUDSTONE.
- Groundwater is presently found between 5 and 9m BGL. As such, excavations deeper than this must be sufficiently dewatered as a mitigation measure to prevent groundwater infiltration and reduce the risk of slope instability.
- Validation of the foundation should be completed by an experienced geotechnical engineer.
- After selection of the foundation system, it is recommended to carry out a settlement analysis to confirm the total and differential settlements are within the tolerance.

We recommend that ADE or a suitably qualified geotechnical engineer be involved during the design stage and construction stage. This is to assist and discuss if the geotechnical recommendations provided in this report is interpreted and implemented effectively in the proposed engineering design with the nominated Structural Engineer and construction contractor.



6 References

- 1:100 000 Newcastle Geological Sheet 9232, first edition from Geological Survey of NSW (1983), Geological Survey of New South Wales, Sydney
- B.G Look Handbook of Geotechnical Investigation and Design Tables (Pub 2007)
- eSPADE 2024, (<u>https://www.environment.nsw.gov.au/eSpade2Webapp/</u>)
- Standards Australia, Australian Standards (AS) 1726-2017 Geotechnical Site Investigations (Pub 2nd May 2017)
- Standards Australia, Australian Standards (AS) 2870-2011 Residential slabs and footings. (Pub 17th January 2011)
- Standards Australia, Australian Standards (AS) 2159-2009 Piling Design and installation. (Pub 4th November 2009)
- Standards Australia, Australian Standards (AS) 3798-2007 Guidelines on earthworks for commercial and residential developments. (Pub 12th March 2007)
- P.J.N. Pells, G. Mostyn and B.F. Walker, Foundations on Sandstone and Shale in the Sydney Region, Australian Geomechanics, December 1998, pp 17-29
- Pells, P.J.N, Douglas D.J, Rodway, B, Thorne C, McManon B.K Design Loadings for Foundations on Shale and Sandstone in the Sydney Region. Australian Geomechanics Journal, 1978Guide to Residential Slabs and Footings in Saline Environments CCAA T56-2005



7 Limitations

This report has been prepared for use by the Client who has commissioned the works in accordance with the project brief only and has been based on information provided by the Client. The advice herein relates only to this project and all results, conclusions and recommendations made should be reviewed by a competent and experienced person with experience in geotechnical investigations, before being used for any other purpose.

ADE Consulting Group Pty Ltd (ADE) accepts no liability for use or interpretation by any person or body other than the client who commissioned the works. This report should not be reproduced or amended in any away without prior approval by the client or ADE and should not be relied upon by any other party, who should make their own independent inquiries.

This report does not provide a complete assessment of the geotechnical status of the site, and it is limited to the scope defined herein. Should information become available regarding conditions at the site (e.g., conditions exposed at the site during earthworks varying significantly with the results within this report), ADE reserves the right to review the report in the context of the additional information.

ADE's professional opinions are based upon its professional judgment, experience, training, and results from analytical data. In some cases, further testing and analysis may be required, thus producing different results and/or opinions. ADE has limited investigation to the scope agreed upon with its client.

This report has been written with the intent of providing information on the site subsurface to the client for design and construction purposes. Subsurface conditions relevant to the works undertaken by the client should be assessed by a competent contractor who can make their interpretation of the data represented within this report.

Subsurface conditions will always vary within a worksite and the extremes of these variations cannot be defined by exhaustive investigations, and as such, the measurements and values obtained within this result may not be representative of these extremes.

Appendix I – Borehole Location Plan

T. 1300 796 922 | E. info@ade.group

This report is copyright. No part of this publication may be reproduced or transmitted in any form or by any means, electronic or mechanical, including photocopying without permission in writing from ADE Consulting Group Pty Ltd.





Appendix II – Borehole Logs and Explanatory Notes

T. 1300 796 922 | E. info@ade.group

This report is copyright. No part of this publication may be reproduced or transmitted in any form or by any means, electronic or mechanical, including photocopying without permission in writing from ADE Consulting Group Pty Ltd.







	25		AD CONSUL GRO	TING			: S	INSV	N-CORE DRILL HOLE - GEOLOGICAL L N PROJECT : Medowie High School Geoter	OG chnica		DLE NO : BH202 FILE / JOB NO : STUGHEET : 4 OF 4
ł	POS		J·F	- 392								ROM HORIZONTAL : 90°
- H		TYPE					DUNTIN			7110		LLER : Toby
İ	DAT	E ST/	ARTE	D: 1	9/11/202				TED : 20/11/2024 DATE LOGGED : 20/11/2024 LOGGED B	Y : C		CHECKED BY :
ľ												
				RILLIN				r	MATERIAL			
ŀ	& CASING 80	WATER	DRILLING PENETRATION	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	GROUP SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations
ł				-		-24.0 -8.9	6/2		Clayey GRAVEL: fine to coarse grained, subrounded to rounded,		-	RESIDUAL SOIL
							198	GC	orange-brown, low to medium plasticity.	w		-
							19					-
								1	24.50m Silty CLAY: low to medium plasticity, pale grey.			
							$\exists /$					-
							Ħ/					-
						25.0 -9.9	$\equiv //$]				_
						-9.9	$\exists /$	CL-CI				-
							$\exists /$					
	8						\equiv					
	— WB		F				\equiv	1				-
									25.80m Sandy CLAY: low plasticity, pale grey, fine to medium grained.	w ≈ PL	н	-
						26.0 -		sc				_
						-10.9	\///		26.25m			-
5									Silty CLAY: medium to high plasticity, pale grey mottled orange-brown.			
023-12						-						-
00.02												-
ADE 2								СІ-СН	1			-
01 Prj:						27.0 -						
023-12	V					-11.9			27.20m			
2.00.0 2									Hole Terminated at 27.20 m			
: ADE 2												-
9												-
- D0							-					-
Situ To						28.0 -	-					_
o and Ir						-12.9						-
tgel Lat												
09 Da												-
0.03.00							1					-
0:56 1							-					-
72024 1						29.0 -	4					-
 20/12 						-13.9						
gFile>>							1					-
Drawin						·	1					-
ěPJ v						· ·	1					-
						.	-					-
IGATIC						30.0 -	4					-
NVES						-14.9						
MBER							1					-
NOVE						· ·	1					-
DOWE						· ·	1					-
ZA ME						.	4					-
HOLE						31.0 -	4					-
BORE						-15.9						-
IS AU							1					-
B Log						· ·	1					-
-319.G						.	1					-
APTOF						.	-					-
B-ADEL					- 6-	32.0 -						
8	detai	Explai ils of a	bbrev	iation	s	-16.9						
ADE 2	& ba	sis of	descr	ptions	6.							





File: BH203 2 OF 2



Datgel Lab and In Situ Tool 10:56 NVESTIGATION.1 1.GPJ REHOLE 2A



^{0.03.00.09} 20/12/2024 10:56 INVESTIGATION.1 1.GPJ NOVEMBER MEDOWIE **OREHOLE 2A** GLB

3	5		ad Consul Gro	TING		LIENT DCATIO	:	SINS	N-CORE DRILL HOLE - GEOLOGICAL I W PROJECT : Medowie High School Geote ndance Road, Medowie NSW 2318	_OC		DLE NO : BH205 FILE / JOB NO : estigatet : 3 OF 8
PC	DSIT	FION	I : E	: 392	786.00, 1	N: 6379	9899.0	00 (M	SA2020-56) SURFACE ELEVATION : 14.61 (AHD)	ANG	GLE F	ROM HORIZONTAL : 90°
				C 450					Track CONTRACTOR : Terratest		DR	ILLER : Toby
D/	ΥE	STA	ARTE	D: 2	5/11/202	4 DAT	ECO	MPLE	TED : 27/11/2024 DATE LOGGED : 27/11/2024 LOGGED E	3Y : (СН	CHECKED BY :
\vdash									MATERIAL			
PR	OGR	ESS								1	×	
DRILLING		WATER	DRILLING PENETRATION	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC I OG	GROUP	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations
Ē	~				SPT	16.0 — -1.4		Π	Silty CLAY: medium to high plasticity, pale grey mottled yellow-brown.			RESIDUAL SOIL
					5.6.8 N=14 16.45m SPT 5.6.8			CI-C	17.50m Silty CLAY: medium to high plasticity, orange-brown mottled pale grey,	-	St	
U 2023-12-41 FTJ; AUE 200.0 2023-12-01					5.6.8 N=14 17.95m SPT 14,19,10/70 HB N=R	- - 18.0 -3.4 - - - - - - - - - - - - - - - - - - -		CI-C	with gravel, with extremely weathered clay bands.	w ≈ PL		
и ««Игампартие»» дил и изиде ни тоо пилонили вадел царата позан тоот - вое и цар. Ами д. ими. 			F		9.37m SPT 6.11,15 N=26 20.95m	20.0		CL-C	19.50m Sandy CLAY: low to medium plasticity, pale grey, medium to coarse grained, with river gravel.		VSt	
LAFTOPRISHED US IS AUDOMENOLE ZA MELUWIE NOVEMBER INVESTIGATION LI 1:512					SPT 7.8,11 N=19 22.45m 22.45m SPT 6.7.9 N=16	- 22.0 -7.4 - - - - - - - - 8.4 - - - - - - - - - - - - -		SC CH	22.00m Clayey SAND: fine to coarse grained, pale grey, low to medium plasticity, with subangular gravel. 23.50m Silty CLAY: high plasticity, pale grey-pale yellow, with trace gravels.	W	MD	
Se	e Ex	xplar	natorv	Note	23.95m s for	24.0						
de	tails	of a	bbrev	iation: ptions	s							

3			TING		LIENT DCATIO	: :	SINSV	I-CORE DRILL HOLE - GEOLOGICAL L / PROJECT : Medowie High School Geote idance Road, Medowie NSW 2318	_OC		DLE NO : BH205 FILE / JOB NO : estigation SHEET : 4 OF 8
PC	SITIC	DN :E	E: 392	2786.00, I	N: 6379	9899.0	0 (MG	A2020-56) SURFACE ELEVATION : 14.61 (AHD)	ANG	GLE F	ROM HORIZONTAL : 90°
		PE:M				UNTI					ILLER : Toby
	TE S	TARTE	D: 2	25/11/202	4 DAT	E COI	MPLE	TED : 27/11/2024 DATE LOGGED : 27/11/2024 LOGGED E	3Y : (СН	CHECKED BY :
\vdash		DF	RILLIN	IG				MATERIAL			
PR	OGRES				ନିର୍ଭ	0			шz	<u>ک</u>	
DRILLING	& CASING WATER	DRILLING PENETRATION	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	GROUP SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTUR	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations
		PRET	GROUND GROUND	SPT 6.9,11 N=20 25.45m SPT 7,10,14 N=24 26.95m SPT 5.8,10 N=18 28.45m SPT 28.45m SPT 28.45m	La S 24.0			26 OW Yes Condary and Minor Components Sitty CLAY: high plasticity, pale grey-pale yellow, with trace gravels. (continued) Sitty CLAY: low to medium plasticity, pale grey mottled yellow brown Sitty CLAY: low to medium plasticity, pale grey mottled red-brown.			& Other Observations
de	tails of	anatory abbrev of descr	viation	S	32.0 — -17.4	<u> </u>	<u> </u>				,

ſ	Ś		CONSUL GRO	TING			: S	INSV	I-CORE DRILL HOLE - GEOLOGICAL L / PROJECT : Medowie High School Geote dance Road, Medowie NSW 2318	LOC		DLE NO : BH205 FILE / JOB NO : estigation SHEET : 5 OF 8
ł	POS	SITION	N : E	: 392					A2020-56) SURFACE ELEVATION : 14.61 (AHD)	ANC	GLE FI	ROM HORIZONTAL : 90°
	RIG	TYPE	E : M	C 450)	MC	UNTIN	G : '	Frack CONTRACTOR : Terratest		DR	ILLER : Toby
	DAT	E ST	ARTE	D: 2	25/11/202	4 DAT	ECON	IPLE	ED : 27/11/2024 DATE LOGGED : 27/11/2024 LOGGED E	BY : (СН	CHECKED BY :
┢					10							
	PROC	RESS		SILLIN S					MATERIAL		≻	
H	& CASING		DRILLING PENETRATION	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	GROUP SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations
ľ						-32.0	\exists		Silty CLAY: low to medium plasticity, pale grey mottled red-brown.			RESIDUAL SOIL
APTOP319G.BL log IS A BOREHOLE ZA MEDOWE NOVEMBER INVESTIGATION 11.169-1 <-CUAMOPFINE 20172024-1035 1003:00/9 Bagei Lab and InStu Tool - DGD Lex ADE 2.000/2023-1201 PF, ADE 2.000/2023-1201		WATE	H DRI		SPT 10,16,23 N=39 35.45m	- 32.0			· · ·		VSt	
					L	40.0	\Box					
ŝ	deta	Expla ils of a isis of	abbrev	iation	s	-25.4						

	Ŕ		AD CONSUL GRO	TING		LIENT DCATIO	:	SINS	I-CORE DRILL HOLE - GEOLOGICAL L W PROJECT : Medowie High School Geote Indance Road, Medowie NSW 2318	_OC		DLE NO : BH205 FILE / JOB NO : estignment : 6 OF 8
	POS	ITION	N : E	: 392					A2020-56) SURFACE ELEVATION : 14.61 (AHD)	ANG	GLE FI	ROM HORIZONTAL : 90°
		TYPE					UNTI					ILLER : Toby
Ľ	DAT	E STA	ARTE	D: 2	25/11/202	4 DAT	ECO	MPLE	TED : 27/11/2024 DATE LOGGED : 27/11/2024 LOGGED B	Y:(СН	CHECKED BY :
┢							<u> </u>					
		RESS		RILLIN ∝					MATERIAL		≻	
H	& CASING	WATER	DRILLING PENETRATION	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	GROUP SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations
ľ	± ∝̃	>	E	ö	아 문 SPT				Silty CLAY: low to medium plasticity, brown, extremely weathered		õ	RESIDUAL SOIL
L					SPT 13,21,17/40r HB	mm ^{-25.4}	Ę٧	2	siltstone vertical bands. (continued)			
L					N=R 40.34m		E/	1				
L						-	E/					-
L						-	Ħ۷	7				-
L							E/	1				-
L						44.0	\equiv					
L						41.0 — -26.4	Ħ۷.	2				
L						-	Ħ٧	1				-
L						-	Ē/	2				-
L							Ħ⁄	2				
						-	\exists					-
						-	É.	2				-
						42.0	F/	1				-
						-27.4	JĒ/					
							Ħ۷	2				
0-21-0						-	E	1				-
202.0.1						-	E/	3				-
AUE 2.00.0 2023-12-01							Ę٧					-
FIJ: AI							E/	1				
10-21-2202						43.0 — -28.4	Ħ٧	2				-
5772 D						-	Ħ٧	2				-
7.00.2	WB -		н				\exists	CL-C		w ≈ PL	н	-
HUE :							Ħ⁄	2				
2						-	F/	1				-
- 100						-	\exists					-
Id IN SILU 1001 - DGU LID: ADE						44.0 —	Ę٧					_
						-29.4	E/	1				
Datgel Lab						-	Ħ٧	7				-
						-	Ħ۷.	2				-
80.00.00.01							\exists	1				-
							Ħ⁄	2				
2/2024 10:00						-	Ħ⁄					-
102/2						45.0	\neq	1				-
'N7 <<						-	₽́/					-
vingrik						.	<u>ا</u>					-
< <p>S<dtaw< p=""></dtaw<></p>							Ħ٧					
v G-D-						-	Ħ/					-
- 1						-	Ħ٧.	1				-
5						46.0	Ħ/	1				-
NVEO						-31.4	JÞ/					
N DER							Ħ⁄	7				-
NOVER						-	Ħ	1				-
						-	\≠					-
	1					-	Ē	4	46.73m Continued as Cored Drill Hole	-		-
715 27						47.0						
						47.0 — -32.4	1					-
						-	1					-
						-	-					-
ALIO 1313.GLD						-	1					-
						-	1					-
	Sec.	Evolor	nator	Note	e for	48.0						
3	detai	Explar ils of a	bbrev	iation	s	-33.4						
AUEZ	& ba	sis of o	descri	ptions	з.							

								ELEVATION				OM HORIZONTAL : 90°								
_		E : M					G : Track			DR : Terratest		ER : Toby								
		arte Diame			4 DAT		PLETED : 27/11/2024 DATE LC RREL (Length) : BIT :	OGGED : 27	/11/2	2024 LOGGE	D BY : CH	CHECKED BY : ONDITION :								
-0	INGL					57	MATERIA				Bir C	FRACTURES								
OG	RESS				(c	0	DESCRIPTION		U Z	ESTIMATED STRENGTH Is(50)		ADDITIONAL DATA								
& CASING	WATER	표절 (CORE LOSS 코티 RUN %)	RQD (%)	SAMPLES & FIELD TESTS	(m) HITABU (m) 40.0	GRAPHIC LOG	ROCK TYPE : Colour, Grain size (texture, fabric, mineral composition alteration, cementation, etc as a	n, hardness	WEATHERING		NATURAL FRACTURE (mm)	 (joints, partings, seams, zones, e Description, orientation, infilling or coating, shape, roughness thickness, other 								
					43.0															
					- 45.0 — - -															
					- 46.0 — - -															
			27.47		_		46.73m START CORING AT 46.73m MUDSTONE: brown, low strength, I	nighly	нw			J— EW Seam, clay ── JT, 0°, CN, UN, RF								
				Is(50) A=0.290 MPa Is(50) D=0.190 MPa	47.0 —		weathered.	 -				JT, 0°, CN, UN, RF]JT, 85°, clay CN, PR, VR ⊐EW Seam, clay ⊐EW Seam, clay JT, 40°, EW Rock CN, IR, RF								
		47.64	80	Is(50) A=0.280 MPa Is(50)			47.67m MUDSTONE: dark grey, low to mee highly weathered to moderately we	lium strength, athered.	HW to MW			— JT, 45°, EW Rock CN, IR, VR — JTEW Rock, 90°, clay CN, PR, RF — DB								
	CLIENT : SINSW LOCATION : 6 Abundan							INSW	CORED DRILL HOLE LOG PROJECT : Medowie High School Ger Road, Medowie NSW 2318							HOLE NO : BH205 FILE / JOB NO : eotechnical Investigation HEET : 8 OF 8				
--	--	--------	------------	------------------------------	------------------------------------	-----------------------	----------------	---	---	------------------------------	----------------	------	--------------------------	------------------	------------	--	----------------------	-----------	--------	---
ł	POS		N : E	: 392				(MGA2020-56)		E ELEVATION	I :1	4.6	1 (A	HD)		A	NGLI	E FF	ROM	HORIZONTAL : 90°
- H				C 450				G: Track		CONTRA	ACTO	DR	: Te	errate	est			DRII	LLEF	R : Toby
- H						4 DAT		IPLETED : 27/11/		OGGED : 27	/11/2	2024	1 L	LOG	GEI) BY				CHECKED BY :
┢	CAS	ING [BA	ARREL (Length) :	BIT						_			BIT		NDITION : FRACTURES
ļ	POG	PESS							MATERI	AL	U	ESTI	MATED Is(5	STREN	IGTH				- 1	ADDITIONAL DATA
	& CASING	WATER	TIT RUN %)	RQD (%)	E SAMPLES &	(m) HLd JD 48.0	GRAPHIC LOG	DI ROCK TYPE : 0 (texture, fabric, m alteration, cem	ineral composit	ion, hardness			ls(5 ● - A O - Dia	Axial Imetral		FRA (r	URAL CTURE nm)		VISUAL	(joints, partings, seams, zones, etc) Description, orientation, infilling or coating, shape, roughness, thickness, other
				80	MPa	-		MUDSTONE: c highly weather (continued)	lark grey, low to me ed to moderately w	edium strength, eathered.	HW to MW								-	DB
					1-(50)	-														DB
	НQ3 —				ls(50) D=0.210 MPa Is(50)	-													-	JT, 5°, CN, PR, RF
	Ī				A=0.160 MPa	49.0 —														— DB — ⊐∕_ JT, 30°, PI, RF
					ls(50) D=0.420	-													-	DB
	V		49.74		MPa Is(50) A=0.490 MPa	-		49.74m												— JT, 30°, clay, PI, RF
						50.0 —		Hole Terminate	ed at 49.74 m											_
						-														-
0.0 2023-12-01						-														-
01 Prj: ADE 2.0						51.0 — -									 		 	 		-
2.00.0 2023-12-						-														-
Tool - DGD Lib: ADE 2.00.0 2023-12-01 Prj: ADE 2.00.0 2023-12-01						- 52.0 —														-
						-														-
2024 14:29 10.03.00.09						- 53.0 — -														-
00.0 LIB.GLB Log IS AU CORED BOREHOLE 2A MEDOWIE NOVEMBER INVESTIGATION.1.GPJ < <drawingfile>> 11/12/2024 14:29 10.03.00.09 DatgeLtab and In Situ</drawingfile>						-														-
BER INVESTIGATION.1.G						54.0 — - -														-
MEDOWIE NOVEM						- - 55.0 —														-
BOREHOLE 2A						-									 					-
29 ISAU COREC						-									- 					-
GLB L						56.0														
	detai	s of a	abbrev	Notes viations iptions	;	00.0														











	2 C		AD CONSUL GRO	TING	CL		: 5	SINSV	-CORE DRILL HOLE - GEOLOGICAL I PROJECT : Medowie High School Geot dance Road, Medowie NSW 2318	LOC echnic		DLE NO : BH208 FILE / JOB NO : estigation SHEET : 3 OF 8
t	POS	ITION	N : E	: 392					A2020-56) SURFACE ELEVATION : 14.25 (AHD)	ANG	GLE FI	ROM HORIZONTAL : 90°
- H		TYPE					UNTIN					LLER : Toby
	DAT	E ST/	ARTE	D: 2	21/11/202	4 DAT	ECO	MPLE	ED : 25/11/2024 DATE LOGGED : 25/11/2024 LOGGED E	BY : (СН	CHECKED BY :
┢					1G				MATERIAL			
F	ROG	RESS	UN UN	TER	s & sts	ÊĴ	U			щZ	Υ E C V	
	& CASING	WATER	DRILLING PENETRATION	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	GROUP SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTUR	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations
ſ					SPT 5,7,11 N=18	16.0			Silty CLAY: medium to high plasticity, yellow-brown. (continued)			RESIDUAL SOIL 16.00: PP =300 - 400 kPa
					N=18	-		сі-сн				-
					16.45m	-			16.50m			-
							\square		Silty CLAY: medium to high plasticity, pale grey.	-		_
						-						-
						17.0 —						_
						-2.8						_
						-						-
					SPT 5,7,10 N=17	-						17.50: PP =300 - 350 kPa
					N=17	-						-
					17.95m							
						18.0 — -3.8						-
						-						-
12-01						-						-
2023-								CI-CH				_
ADE 2.00.0 2023-12-01												
						-						-
2-01 Prj					SPT	19.0 — -4.8						19.00: PP =250 - 250 kPa
2.00.0 2023-12-01					SPT 4,7,8 N=15	-4.0						-
00.00												
ADE					19.45m							-
Ë						-						-
I - DGD						-						-
situ Toc	WB		F			20.0 —				w ≈ PL	VSt	_
nd In Situ	≥		г			-5.8				W-PL	. vsi	_
Lab a						-						-
Datgel						-			70 F0			-
10.03.00.09					SPT	1_	M		20.50m Silty CLAY: medium to high plasticity, grey mottled yellow-brown.	-		20.50: PP =550 - 450 kPa
					5,9,12 N=21							
4 10:56					20.95m	-						-
12/202+						21.0 — -6.8						
>> 20/						-						-
ingFile												
< Draw						-						-
GPJ <<						-						-
N.11.						-						-
IGATIC					ODT	22.0		СІ-СН				_
NVEST					SPT 8,12,17 N=29	-22.0						
						-	\mathbb{M}					-
NOVEMBER					22.45m	-						-
						-						-
MEDOWIE						_						-
ILE 2A												
AU BOREHOLE						23.0 — -8.8						-
						-						-
'SI Bo-						-			20.50			-
GLBL					SPT	1	⊬ ∦		23.50m Silty CLAY: medium to high plasticity, pale grey.	-		23.50: PP =400 - 350 kPa
0P319					3,8,11 N=19	-		сі-сн				-
ELAPT(23.95m	-		U-UH				-
IB-ADI	 See	Explar	natore	Noto		24.0	r⊿∐	1				
8	detai	ils of a	bbrev	iation	S	-9.ð						
ADE 2	& ba	sis of (descri	ption	S.							

	2			LTING	CL	LIENT	: 8	SINSV	I-CORE DRILL HOLE - GEOLOGICAL	L OC echnic		DLE NO : BH208 FILE / JOB NO : estigater : 4 OF 8
ľ	PO	SITIO	N : E	E: 392					A2020-56) SURFACE ELEVATION : 14.25 (AHD)	ANG	SLE FI	ROM HORIZONTAL : 90°
	RIG	TYP	E : M	IC 450)	MO	UNTIN	G :	Track CONTRACTOR : Terratest		DR	ILLER : Toby
	DA	TE ST	ARTE	D:2	1/11/202	4 DAT	E CON	/IPLE	TED : 25/11/2024 DATE LOGGED : 25/11/2024 LOGGED I	BY : (СН	CHECKED BY :
				RILLIN		1		-	MATERIAL	-		
- F		GRESS	DRILLING PENETRATION	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	GROUP SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations
ł				-		-24.0 -9.8	Þπ		Silty CLAY: medium to high plasticity, pale grey. (continued)		-	RESIDUAL SOIL
- F			OLITINO F BUILLING	GROUNDWATE	© SPT SPT 8,13,17 N=30 25.45m SPT 8,16,18 N=34 26.95m SPT 12,17,20 N=37 28.45m SPT 12,17,20 N=37 28.45m	- 24.0		CL-CL CL CL-CL CL CL-CL CL C	Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	Moisture Condition	н сonsistency и consistency и consistency и consistency и сонзистенся и сонзистенся и сонзистенся и сонзистенся	
							$\exists /$	1				
P319.6						-	Ħ⁄/					-
_APTOP319.GLB						-	$\exists /$					-
ADELA						32.0	EV					
ADE 2.00.0 LIB-	deta	ails of a	anatory abbrev descr	/iation	s	-17.8						

	8		CONSUL GRO	TING		LIENT DCATIO	: S	INSV	I-CORE DRILL HOLE - GEOLOGICAL I / PROJECT : Medowie High School Geote idance Road, Medowie NSW 2318	_OC		DLE NO : BH208 FILE / JOB NO : estiggtion HEET : 5 OF 8
									A2020-56) SURFACE ELEVATION : 14.25 (AHD)	ANG		ROM HORIZONTAL : 90°
				C 45					CONTRACTOR Terratest FED : 25/11/2024 DATE LOGGED : 25/11/2024 LOGGED E	3Y : C		LLER : Toby CHECKED BY :
							1					
	OGR			RILLIN □ ∝					MATERIAL		>	
		WATER	DRILLING PENETRATION	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	GROUP SYMBOL	MATERIAL DESCRIPTION Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOISTURE CONDITION	CONSISTENCY RELATIVE DENSITY	STRUCTURE & Other Observations
						-17.8			Silty CLAY: low to medium plasticity, pale grey mottled orange-brown. (continued)			RESIDUAL SOIL
								CL-CI		w ≈ PL	н	-
							269		32.50m Clayey GRAVEL: fine to coarse grained, subrounded, pale grey, low to			-
						- 33.0 — - ^{18.8} -		GC	medium plasticity.	w	VD	-
						-						-
						-			24.00m			-
						34.0 — -19.8			34.00m Silty CLAY: low to medium plasticity, pale grey mottled orange-brown.			-
						-	Ē/					-
0-21-620						-						-
7.00.2						-	\exists					-
JI: ANE						-	E/					-
10-71-6						35.0 — -20.8	$\exists /$	1				-
חיח לחלי						-	\equiv	1				-
AUE 2.1						-	Ħ٧	1				-
20						-	E/]				-
- 100						-	\equiv					-
			н			36.0 — -21.8						-
						-	\equiv					-
a naige						-	E/					-
60.00.00.0						-	ŧ/					-
8						-	E/					-
47077						37.0		CL-CI		w ≈ PL	н	-
107 ~~						-22.8						-
an guiden						-	\equiv					-
						-	$\equiv /$	1				-
2						-		1				-
						38.0 — -23.8	$\exists /$	1				_
IN EQ						-23.8	\neq					-
						_	\equiv					-
						.						-
MEDO						-	$\exists /$					-
						39.0 —		1				_
						-24.8	/	1				-
e for						-	 /]				-
3.0LD						-	티/					
2012						-	Ē/					-
					<u> </u>	40.0	Ē/	1				
de	tails	of a	bbrev	v Note viation iption	IS	-25.8						

ſ	2		CONSU GRO	LTING	CI		: S	INSV	I-CORE DRILL HOLE - GEOLOGICAL L / PROJECT : Medowie High School Geoter idance Road, Medowie NSW 2318	OC chnic		DLE NO : BH208 FILE / JOB NO : estigation: SHEET : 6 OF 8
ł	POS	SITION	N : E	E: 392						ANG	GLE FI	ROM HORIZONTAL : 90°
- H		TYPE					UNTIN					ILLER : Toby
	DAT	E ST	ARTE	D:2	1/11/202	4 DAT	E CON	1PLE	TED : 25/11/2024 DATE LOGGED : 25/11/2024 LOGGED B	Y:(СН	CHECKED BY :
┟				RILLIN		1			MATERIAL			
- F		GRESS	ING	WATER	LES & TESTS	H (m) AHD)	UHC HC	an P	MATERIAL DESCRIPTION	'URE	TENCY TIVE SITY	STRUCTURE
	& CASING	WATER	DRILLING	GROUND WATER LEVELS	SAMPLES & FIELD TESTS	DEPTH (m) RL (m AHD)	GRAPHIC LOG	GROUP SYMBOL	Soil Type, Colour, Plasticity or Particle Characteristic Secondary and Minor Components	MOIS ⁻	CONSISTENCY RELATIVE DENSITY	& Other Observations
ł			-			40.0	ار ا		Silty CLAY: low to medium plasticity, pale grey mottled orange-brown.			RESIDUAL SOIL
						-	$\exists \prime$	1	(continued)			-
						-	\exists					_
							\equiv	1				
						-	$\exists /$	1				-
						-	$\equiv /$					-
						41.0 -	$\equiv /$	1				_
						-26.8	$\exists /$					
							\exists					
						-	$\exists /$					-
						-	$\exists /$	1				-
						.	\neq					_
							$\not\models$	CL-CI		w ≈ PL	н	
						42.0 — -27.8	/	1		/ 6		-
	8		н			-	$\exists /$					-
2-01	— WB						$\exists /$	1				_
2023-1							$\exists /$	1				
2.00.0						-	$\exists /$					
ADE						-	$\equiv /$	1				-
01 Prj						43.0 -	$\exists /$					_
023-12						-28.8	$\equiv /$					
00.0 20							$\exists /$	1				
VDE 2.0						-	$\exists /$					-
LIB:						-	\equiv					-
- DGD							$\exists /$	1				_
u Tool									43.90m Clayey SAND: fine to coarse grained, grey, low to medium plasticity.			
d In Sil						44.0 — -29.8	////		Clayey ORIVD. The to coarse graned, grey, low to medium plasticity.			
Lab an						-	\././.	sc		w	VD	-
Datgel				-		.		1				_
60.00	¥						·/././.		44.60m			
10.03.									Continued as Cored Drill Hole			
4 10:57						-	1					-
2/202						45.0 — -30.8	1					-
> 20/						-30.8	-					-
ingFile:												
Drawi						-]					
GPJ ¢						-	1					-
N 11						-	-					-
IGATIC						46.0-						_
WEST						46.0 — -31.8						
BER						-]					-
OVEM						-	1					-
MEN						-	-					-
MEDC						.]					
LE 2A												
REHC						47.0 — -32.8	1					-
AUBC						-	1					-
S Bo						-	-					-
GLB												
0P319.						-	1					-
ELAPT(-	1					-
B-ADE	S00	Expla	nator	(Note	s for	48.0						
8	deta	ils of a	abbrev	/iation	s	-33.8						
ADE 2	& ba	isis of	descr	ptions	3.							

			E: 392				(MGA2020-56) G : Track	SURFACE ELEVATIO		4.25 (AHD) DR : Terrate			/ HORIZONTAL : 90° R : Toby
							PLETED : 25/11/2024	DATE LOGGED : 2			GED BY : CH		CHECKED BY :
AS	ING [DIAM	ETER	:		BA	RREL (Length) :	BIT :			E	зіт со	NDITION :
			RILLIN		1			MATERIAL					FRACTURES
	RESS	() ()	(%	ES & ESTS	(L)	U HC	DESCR ROCK TYPE : Colour		RIN	ESTIMATED STRENG Is(50) • - Axial • - Diametral	NATURAL	. T	ADDITIONAL DATA (joints, partings, seams, zones, et
& CASING	WATER	E (CORE LOSS HIT RUN %)	RQD (%)	SAMPLES & FIELD TESTS	(W) HLAJO .0.	GRAPHIC LOG	(texture, fabric, mineral alteration, cementatio	composition, hardness	WEATHERING	O-Diametral	FRACTURE (mm)	<pre> Alsi </pre>	Description, orientation, infiling or coating, shape, roughness, thickness, other
					- - 43.0 — - - - 44.0 — -								
			0		- 45.0 —		44.60m START CORING AT 4 MUDSTONE: grey-bro						
			0				45.51m NO CORE 0.36m (45	51-45.87) CORE LOSS				_	— EWZ
						X			X				
		45.87 45.94	\vdash		40.5		45.87m MUDSTONE: grey-bro	own	$\left\{ \right\}$			-	
			44		46.0 —								
				1.000	-		46.31m		$\left \right $				JT, 10°, clay CN, PR, RF
				Is(50) A=0.0400 MPa			MUDSTONE: dark gre	·7·					→ JT, 20°, rock CN, PR, RF → JT, 20°, clay CN, PR, RF → JT, 20°, rock CN, PR, RF
				Is(50) D=0.0400 MPa									JT, 20°, rock CN, PR, RF JT, 20°, rock CN, PR, RF CZ
				Is(50) A=0.0400 MPa									JT, 10°, rock CN, PR, RF DB
				ls(50) D=0.0500 MPa	47.0 —								JT, 80°, rock, clay CN, UN, RF
													J JT, 30°, rock CN, PR, RF
				ls(50) A=0.160 MPa	-								JT, 0°, rock CN, PR, RF J-JT, 10°, rock CN, PR, VR
		47.70	93.7	Is(50) D=0.0600 MPa	-								└└JT, 10°, clay CN, PR, VR └─CZ ──JT, 20°, clay CN, UN, RF
			33.1	Is(50) A=0.230 MPa	-								JT, 20 , clay CN, UN, RF

						(MGA2020-56) SURFACE ELEVATION					/ HORIZONTAL : 90°
G TYP								OR : Terratest		ILLE	R : Toby
ATE ST ASING				4 DAI		IPLETED : 25/11/2024 DATE LOGGED : 25 \RREL (Length) : BIT :	5/11/	/2024 LOGGE	D BY : CH		CHECKED BY : NDITION :
						MATERIAL					FRACTURES
& CASING WATER	g (CORE LC	RQD (%)	SAMPLES & FIELD TESTS	DEPTH (m)	GRAPHIC LOG	DESCRIPTION ROCK TYPE : Colour, Grain size, Structure (texture, fabric, mineral composition, hardness alteration, cementation, etc as applicable)	WEATHERING	ESTIMATED STRENGTH Is(50) O - Diametral	NATURAL FRACTURE (mm)	VISUAL	ADDITIONAL DATA (joints, partings, seams, zones, e Description, orientation, infilling or coating, shape, roughness, thickness, other
	DEPTH	93.7	Is(50) D=0.330 MPa Is(50)	48.0		MUDSTONE: dark grey. (continued)	5		20		DB JT, 0°, clay CN, PR, RF
	48.34		A=0.280 MPa			48.40m					JT, 45°, Fe CN, PR, RF
			ls(50) D=0.330 MPa	- - 49.0 —		Hole Terminated at 48.40 m					
				-							
				50.0 — - -							
				- 51.0 —							
				- - 52.0 —							
				- - 53.0 —							
				- - - 54.0 —							
				- - -							
				55.0 — - -							







EXPLANATORY NOTES

Soil and rock descriptions on the logs are generally in accordance with the recommendations of AS1726-2017 Geotechnical Site Investigation.

The order in which descriptions are provided on the logs is as follows:

1. SOIL NAME AND GROUP SYMBOLS

Major Divis	sions	Symbol	Description
reater	oarse nm	GW	Well-graded gravels, gravel-sand mixtures, wide range in grain size and substantial amounts of all intermediate sizes, little or no fines
ion is g	GRAVEL an 50% of c an is >2.36n	GP	Poorly graded gravels, gravel-sand mixtures, predominantly one size or range of sizes with some intermediate sizes missing, little or no fines
LS ed fract	GRAVEL More than 50% of coarse fraction is >2.36mm	GM	With appreciable amount of non-plastic fines, zero to medium dry strength (gravel- sand-silt mixtures)
IED SOI Oversize Simm		GC	With appreciable amount of plastic fines, medium to high dry strength (gravel-sand- clay mixtures)
COARSE GRAINED SOILS soil excluding oversized than 0.075mm	e fraction	SW	Wide range in grain size and substantial amounts of all intermediate sizes, not enough fines to bind coarse grains, no dry strength (Well graded sands, gravelly sands, little or no fines)
COARSE GRAINED SOILS More than 65% of soil excluding oversized fraction is greater than 0.075mm	SAND More than 50% of coarse fraction is <2.36mm	SP	Predominantly one size or range of sizes with some intermediate sizes missing, not enough fines to bind coarse grains, no dry strength (Poorly graded sands and gravelly sands; little or no fines, uniform sands)
chan 65	han 509 is <	SM	With appreciable amount of non-plastic fines, zero to medium dry strength (silty sands, sand-silt mixtures)
More	More t	SC	With appreciable amount of plastic fines, medium to high dry strength (clayey sands, sand-clay mixtures)
of of m	% t d	ML	Inorganic silts of low plasticity (very fine sands, rock flour, sandy clays, silty clays)
5% o SO 5% (ing 1tior 75m	Liquid Limit <50%	CL, CI	Inorganic clays of low to medium plasticity (gravelly clays, sandy clays, silty clays)
E GRAINED SC ore than 35% soil excluding rsized fraction s than 0.075m		OL	Organic silts and organic silty clays of low plasticity
tha tha exc exc zed	6 t d	MH	Inorganic silts of high plasticity
FINE GRAINED SOILS More than 35% of soil excluding oversized fraction is less than 0.075mm	Liquid Limit >50%	СН	Inorganic clays of high plasticity
P A A A A A A A A A A A A A A A A A A A		ОН	Organic clays of medium to high plasticity
	Highly organic soil	PT	Peat muck and other highly organic soils

2. PARTICLE SIZE CHARACTERISTICS

Fraction	Components	Sub Division	Size (mm)
	Boulders		>200
	Cobbles		63 - 200
		Coarse	19 - 63
	Gravel	Medium	6.7 - 19
Coarse		Fine	2.36 - 6.7
grained		Coarse	0.6 - 2.36
soil	Sand	Medium	0.21 - 0.6
	Junu	Fine	0.075 - 0.21
Fine grained	Silt		0.002 - 0.075
soil	Clay		<0.002

3. PLASTICITY PROPERTIES





4. MINOR COMPONENTS

	Coarse Grained Soils		Fine Grained Soils
% Fines	Modifier	% Coarse	Modifier
<5	Omit or use 'trace'	<15	Omit or use 'trace'
5 - 12	Describe as 'with clay/silt' as applicable	15 - 30	Describe as 'with sand/gravel' as applicable
>12	Prefix soil as 'silty/clayey' as applicable	>30	Prefix soil as 'sandy/gravelly' as applicable

5. MOISTURE CONDITION

	Field Identif	ication	
Symbol	Cohesive soils	Symbol	Granular soils
w <pl< td=""><td>Hard and friable – Moisture Content of soils is less than the plastic limit</td><td>D</td><td>No Cohesion, dry to the touch and free running</td></pl<>	Hard and friable – Moisture Content of soils is less than the plastic limit	D	No Cohesion, dry to the touch and free running
w=PL	Feels cool, darkened in colour, can be moulded – Moisture Content is equal to plastic limit	М	Feels cool, darkened in colour, no visible water, tends to cohere
w>PL	Feels cool, darkened in colour, usually soft – Moisture Content is greater than plastic limit	W	Feels cool, darkened in colour, tend to cohere, and visible free water. Usually from below water table

Moisture content of cohesive soils shall be described in relation to plastic limit (PL) for the soils with dry of PL (w<PL); near PL (w=PL); and wet of PL (w>PL). Moisture content of non-cohesive (granular) soils shall be described as dry (D), moist (M), wet (w)

6. DENSITY

Term	Very Loose	Loose	Medium Dense	Dense	Very Dense
Symbol	VL	L	MD	D	VD
SPT (N)	0 - 4	4 - 10	10 - 30	30 - 50	>50
DCP	0 - 1	1 - 3	3 - 8	8 - 15	>15
Density Index (%)	<15	15 - 35	35 - 65	65 - 85	>85

7. CONSISTENCY

Term	Very Soft	Soft	Firm	Stiff	Very Stiff	Hard
Symbol	VS	S	F	St	Vst	Н
SPT (N)	0 - 2	2 - 4	4 - 8	8 - 15	15 - 30	>30
DCP	0 - 1	1 - 2	2 - 3	3 - 7	7 - 12	>12
Undrained Shear Strength (kPa)	<12	12 - 25	25 - 50	50 - 100	100 - 200	>200



ROCK DESCRIPTION - EXPLANATORY NOTES

1. STRENGTH

Term	Log Symbol	Point Load Index IS₅₀ (MPa)	Field Guide
Very Low	VL	0.03 - 0.1	Material crumbles under firm blows with sharp end of pick; can be pealed with a knife; too hard to cut a triaxial sample by hand. SPT will refuse. Pieces up to 3 cm thick can be broken by finger pressure. Sandstone is 'sugary' and friable
Low	L	0.1 - 0.3	Easily scored with a knife; indentations 1 mm to 3 mm show in the specimen with firm blows of the pick point; has dull sound under hammer. A piece of core 150 m long x 40 mm Φ may be broken by hand. Sharp edges of core may be friable and break during handling
Medium	М	0.3 - 1	Readily scored with a knife; piece of core 150 mm long x 50mm Φ can be broken by hand with difficulty
High	н	1 - 3	Can be slightly scratched with a knife. A piece of core 150 mm long x 50 mm Φ cannot be broken by unaided hands but can be broken with a single blow, rock rings under hammer
Very High	VH	3 - 10	Cannot scratch with a knife. Hand specimen breaks with pick after more than one blow, rock rings under hammer
Extremely High	EH	>10	Specimen requires many blows with geo-pick to break through intact material, rock rings under hammer

2. WEATHERING

Classification	Symbol	Description
Residual Soil	RS	Soil derived from the weathering of rock; the mass structure and substance fabric are no longer evident; the soil has not been significantly transported.
Extremely Weathered	xw	Rock is weathered to such an extent that it has 'soil' properties, that is, it either disintegrates or can be remoulded, in water. Fabric of original rock still visible.
Distingthe Weath avail	нw	Rock strength usually changed by weathering. The rock may be highly discoloured, usually be iron staining. Porosity may be increased by leaching or may be decreased due to deposition of
Distinctly Weathered	MW	weathering products in pores. In some environments it is convenient to subdivide into Highly Weathered (HW) and Moderately Weathered (MW), with the degree of alteration typically less for MW.
Slightly Weathered	SW	Rock is partially discoloured with staining along joints but shows little or no change of strength from fresh rock.
Fresh Rock	FR	Rock shows no sign of decomposition or staining.

3. COMMON DEFECTS IN ROCK MASS

Туре	Symbol	Description						
Bedding	Ве	A surface or crack across which the rock has little or no tensile strength. Parallel or sub-parallel to layering. May be open or closed.						
Joint	Jt	A surface or crack with no apparent shear displacement and across which the rock has little or no tensile strength. May be open or closed.						
Shear Zone Sz		Zone of rock material with roughly parallel near planar, curved or undulating boundaries cut by closely spaced joints, shear surface or other defects. Some of the defects are usually curved and intersect to divide the mass into lenticular or wedge-shaped blocks.						
Clay Seam	Cs	Seam of soil material with roughly parallel almost planar boundaries, composed of clay.						
Crushed Seam Zone	Cz	Seam of material with roughly parallel almost planar boundaries, composed of disorientated, usually angular fragment of the host rock, which may be more weathered than the host rock.						
Infilled Seam	Se	Seam of soil material with distinct roughly parallel planar boundaries formed by the migration of soil into an open cavity or joint and must be defined by colour and USC symbols. Infilled seams less than 1 mm thick may be described as a veneer or coating on a joint surface.						
Extremely Weathered Seam / Zone	Ewz	Seam of soil material, often with gradational boundaries. Formed by weathering of the rock material in place.						



4. DEFECTS SHAPES AND CHARACTERISTICS

Characteristics	Description
Thickness	Measured in mm normal to the plane of the defect (mm.t).
Inclination	Measured in an exposure as a dip and strike or dip and dip direction. In core measured as an angle from a plane normal to the core axis.
Surface Shape	 Described defect surface shape as either: Planar (PI) – defect forms a continuous plane without variation in orientation Curved (Cu) – defect has a gradual change in orientation Undulating (Un) – a defect has wavy surface Stepped (St) – a defect has one or more well defined steps. Irregular (Ir) – a defect with many sharp changes of orientation
Surface Roughness	A description of the defect plane described as: - Rough (Ro)– many small surface irregularities. - Smooth (Sm) – smooth to touch. Few or no surface irregularities. - Polished (Po) – shiny or sheen smooth surface inconsistent with parent rock - Slickensided (SI) – Grooved or striated surface, usually polished.
Coating	Described defect coating as either: - Clean (Cl) – no visible coating - Stained (St) – no visible coating but surfaces are discoloured - Veneer (Ve) – a visible coating soil or mineral substance, but usually unable to be measured (usually <1 mm), may be called patchy veneer. - Coating (Co) – a visible coating of soil or mineral up to 1mm thick. Thicker soil materials shall be described using appropriate defect terms (e.g. in-filled seam). Thicker rock strength material shall be described as a vein.
Spacing	Measurement of the distance between defects of the same set.

LOG SYMBOLS AND ABREVIATIONS

1. DRILLING AND EXCAVATION METHODS

HA	Hand Auger	RAB	Rotary Air Blast	NMLC	Diamound Core: 52mm
DT	Diatube Coring	RC	Reverse Circulation	HQ	Diamound Core: 63mm
NDD	Non-destructive Digging	РТ	Push Tube	HMLC	Diamound Core: 63mm
AD	Auger Driling (ADV: V-Bit; ADT: TC-Bit)	СТ	Cable Tool Rig	BH	Tractor Mounted Backhoe
ADH	Hollow Auger	JET	Jetting	EX	Tracked Hydraulic Excavator
RD	Rotary blade or drag bit	WB	Washbore or Bailer	EE	Existing Excavation
RT	Rotary Tricone bit	NQ	Diamound Core: 47mm	HAND	Excavated by Hand Methods



2. GRAPHIC SYMBOL LEGENDS FOR SOIL AND ROCK

SOI	-	ROC	<u>K</u>
	FILL	00	CONGLOMERATE
	TOPSOIL		SANDSTONE
	CLAY (CL, CI, CH)		SHALE/MUDSTONE
	SILT (ML, MH)		SILTSTONE
	SAND (SP, SW)		CLAYSTONE
000	GRAVEL (GP, GW)		COAL
	SANDY CLAY (CL, CI, CH)	Π	LAMINITE
	SILTY CLAY (CL, CI, CH)		LIMESTONE
\square	CLAYEY SAND (SC)		PHYLLITE, SCHIST
	SILTY SAND (SM)		TUFF
	GRAVELLY CLAY (CL, CI, CH)	恣	GRANITE, GABBRO
	CLAYEY GRAVEL (GC)		DOLERITE, DIORITE
	SANDY SILT (ML, MH)	$\langle \rangle \rangle$	BASALT, ANDESITE
444 744 744 744	PEAT AND HIGHLY ORGANIC SOILS (Pt)		QUARTZITE

OTHER MATERIALS



Appendix III – Laboratory Test Results

T. 1300 796 922 | E. info@ade.group

This report is copyright. No part of this publication may be reproduced or transmitted in any form or by any means, electronic or mechanical, including photocopying without permission in writing from ADE Consulting Group Pty Ltd.



ADECONSULTING GROUP

POINT LOAD STRENGTH INDEX REPORT

Client:		SINSW				Job No.		A201024.012	4 01	
Project:		New Meadowie High School GI					Report No.			
						Date Teste		A201024.0124.01_A_v1f		
Location:		6 Abundance Road, Medowie NSW 2318						3/12/2024		
Moisture Content Condition:							Storage History:			
Date Sampled:		25/11/2024				Sampled b	y:		PB	
Test Procedure	•	✓ AS 4133.4.1 Rock strength tests - Determination of point load strength index								
Sampling:	•	AS 4133.4.1 Rock strength tests - Determination of point load strength index BH205								
		B11203				В				
Preparation:						В				
		I		-				r r		
Sample No.	Sample Source (m)	Sample Description	Test Type	Average Width (mm)	Platen Separation (mm)	Failure Load (kN)	Point Load Index Is (Mpa)	Point Load Index Is ₍₅₀₎ (Mpa)	Failure Modes	
			Diametral		60.0	0.67	0.19	0.20	1	
1	46.84	MudStone	Axial	60.0	36.4	0.80	0.29	0.29	5	
				00.0						
2	47.84	MudStone	Diametral		60.0	0.80	0.22	0.24	1	
_		maastone	Axial	60.0	36.4	0.77	0.28	0.28	5	
2	49.67	NAU-JCL	Diametral		60.0	0.53	0.15	0.16	1	
3	3 48.67	MudStone	Axial	60.0	46.8	0.69	0.19	0.21	5	
	1		Diametral		51.1	1.27	0.49	0.49	1	
4	49.26	MudStone		60.0						
			Axial	60.0	49.9	1.46	0.38	0.42	5	
								∟⊺		
					l					
								+ +		
						L		↓		
								<u> </u>		
								++		
								┝───┤		
					İ					
	1							+ +		
								++		
								┝───┤		
Œ			2	Œ		D 3	00)	4	
R.	9	6		2	8		9		10	
<u>Notes:</u>										



ADECONSULTING GROUP

POINT LOAD STRENGTH INDEX REPORT

SP										
uent.		SINSW				Job No.		A201024 012	24.01	
Client:								A201024.0124.01		
Project:		New Meadowie High S				Report No.		A201024.012	4.01_A_v1f	
ocation:		6 Abundance Road, Me	edowie NSW 23	18		Date Teste	d:	3/12/2024		
Moisture Contei	nt Condition:	Moist		-		Storage Hi		, ,		
	ni conunion:									
Date Sampled:		25/11/2024				Sampled b			PB	
Fest Procedure:		1								
Sampling:		BH208								
		BH208								
Preparation:						В				
Sample No.	Sample Source (m)	Sample Description	Test Type	Average Width (mm)	Platen Separation (mm)	Failure Load (kN)	Point Load Index Is (Mpa)	Point Load Index Is ₍₅₀₎ (Mpa)	Failure Modes	
			Diamatural		60.0	0.14	0.04	0.04	1	
1	46.36	MudStone	Diametral					_		
			Axial	60.0	35.4	0.10	0.04	0.04	6	
-	46.55		Diametral		60.0	0.16	0.04	0.05	1	
2	46.69	MudStone	Axial	60.0	40.6	0.11	0.04	0.04	6	
				00.0						
3	47.39	MudStone	Diametral		60.0	0.19	0.05	0.06	1	
5	47.55	muustone	Axial	60.0	38.5	0.46	0.16	0.16	6	
			Diametral		60.0	1.09	0.30	0.33	1	
4	47.79	MudStone	-	60.0						
		Axial	60.0	43.7	0.71	0.21	0.23	5		
_ T	10 05	MudCtore	Diametral		60.0	1.11	0.31	0.33	1	
5	48.05	MudStone	Axial	60.0	43.9	0.89	0.27	0.28	5	
			,		.5.5	0.00	Q.L1	0.20	-	
					ł					
					1			1 1		
								<u> </u>		
		1	1					+ +		
					L					
1			1					1		
								<u> </u>		
					1			1		
					1					
								1		
		1	1		+			+ +		
								I		
			1		İ			1		
				-	+					
								1		
				-	1			+		
								1		
					1			┼───┤		
0		(D		<u>(</u>		D 3	00)	4	
0		0		Œ		D 3	00)	4	
0		CD				D 3	08)	4	
		00				۲ ۲	00)	4	
		CD PA		0 0 2		D ,	00)	4	
		- CD FJ				D 3)	4	
		B	K		S	D 3)		
		D P f		(((D 3)	4 DD 10	
		B	R	0 2	S	₽ ₽)		
-) 1	B	R	Q 2	S	D 3)	BB	
-		B	R	(((S	D 3)	BB	
-		B	R	<u>(</u>	S	D 3)		
) 1	B	R	A	S	J ,)		
-		B	R	2 2	S	2 2	C O C C C C C C C C C C C C C C C C C C)		
-		B	R	A	S	D 3	() (Ö) () () () () () () () () () () () () ())		
-		B	R	2 2	S	۲ ۲				
-		B	R	A	S	2 2	() () () () () () () () () () () () () ()		



Further details regarding ADE's Services are available via

📧 info@ade.group 🕀 www.ade.group

ADE Consulting Group Pty Ltd

Sydney Unit 6/7 Millennium Court, Silverwater, NSW 2128

ADE Consulting Group (QLD) Pty Ltd

Brisbane Unit 10/53 Metroplex Avenue, Murarrie, QLD 4172 Newcastle Unit 9/103 Glenwood Drive, Thornton, NSW 2322

ADE Consulting Group (VIC) Pty Ltd

Melbourne Unit 4/95 Salmon Street, Port Melbourne, VIC 3207

T. 1300 796 922 | E. info@ade.group

This report is copyright. No part of this publication may be reproduced or transmitted in any form or by any means, electronic or mechanical, including photocopying without permission in writing from ADE Consulting Group Pty Ltd.